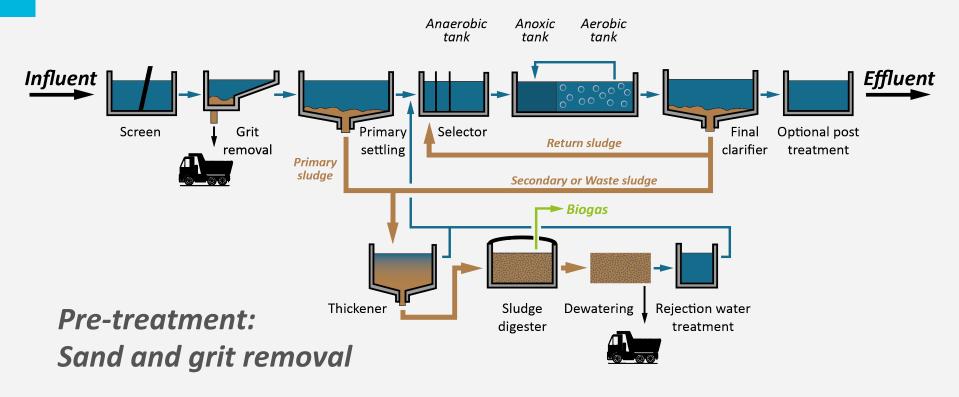
Grit removal design

CTB3365x Introduction to water treatment

Prof.dr.ir. Jules B. van Lier



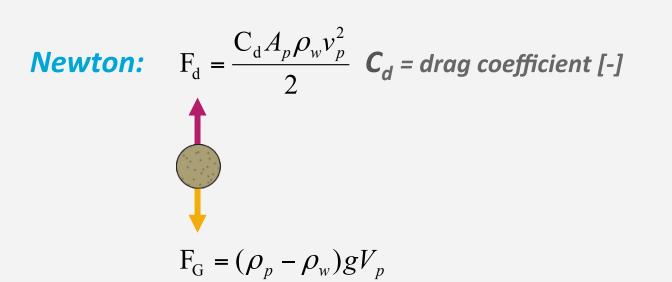
Basic WWTP process units



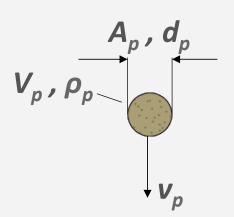
Discrete settling: principles

Settling of discrete, non-flocculating particles:

classical sedimentation laws by Newton and Stokes



Surface area, Diameter:



Discrete settling: terminal particle velocity

Newton's law (spherical particles):

$$F_{G} = F_{d} \qquad A = \frac{\pi d_{p}^{2}}{4} \qquad V = \frac{4}{3}\pi \left(\frac{d_{p}}{2}\right)^{3}$$

$$V = \frac{4}{3}\pi \left(\frac{d_p}{2}\right)$$

$$F_{G} = (\rho_{p} - \rho_{w})gV_{p}$$

$$F_{d} = \frac{C_{d} A_{p} \rho_{w} v_{p}^{2}}{2}$$

$$F_{G} = (\rho_{p} - \rho_{w})gV_{p}$$

$$V_{p} = \sqrt{\frac{4g}{3C_{d}} \left(\frac{\rho_{p} - \rho_{w}}{\rho_{w}}\right)} d_{p}$$

$$F_{d} = \frac{C_{d}A_{p}\rho_{w}v_{p}^{2}}{2}$$
terminal valueity of ρ_{w}

terminal velocity of particle [m/s]

Discrete settling: factors affecting v_p

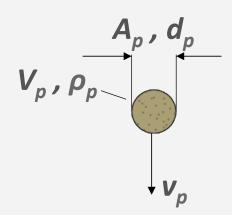
Nonspherical particles:

$$v_{p} = \sqrt{\frac{4g}{3C_{d}\varphi} \left(\frac{\rho_{p} - \rho_{w}}{\rho_{w}}\right) d_{p}}$$

Value C_d depends on flow regime

Sphericity factor φ:

- Spheres: $\phi = 1.0$
- Sand grains: $\phi = 2.0$
- Up to 20+ for fractal flocs



Discrete settling: impact of liquid turbulence

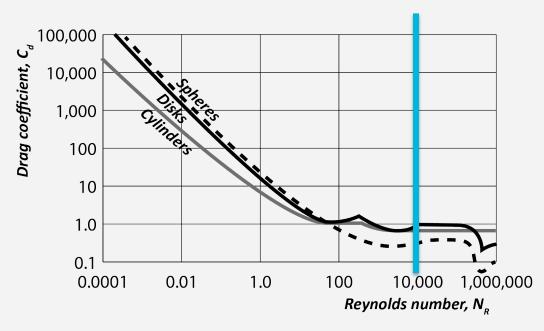
Drag coefficient C_d

$$C_d = \frac{24}{N_R} + \frac{3}{\sqrt{N_R}} + 0.34$$

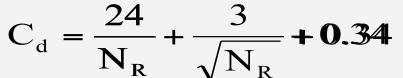
Depends on turbulence

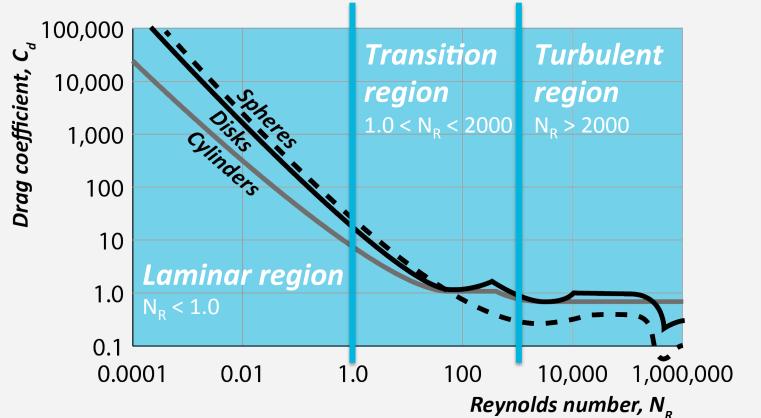
$$N_{R} = \frac{v_{p}d_{p}\rho_{w}}{\mu} = \frac{v_{p}d_{p}}{\nu}$$

 μ = dynamic viscosity [Ns/m²] ν = kinematic viscosity [m²/s]



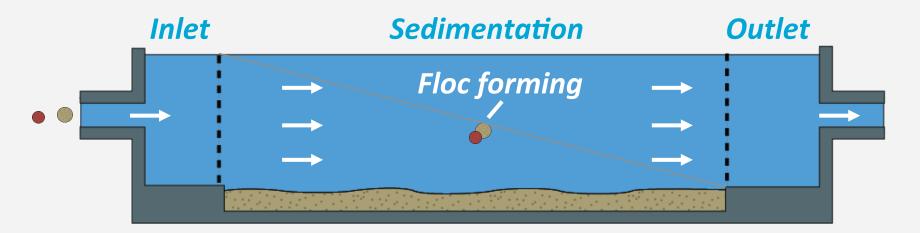




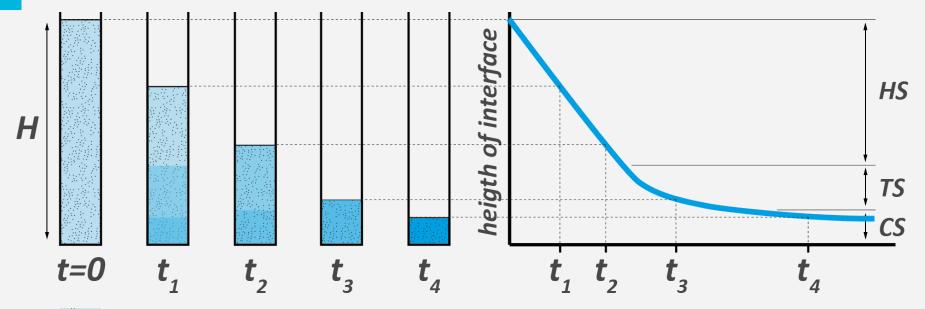


Flocculent settling

- Flocculation causes increasing settling velocities
- Dilute suspension settling

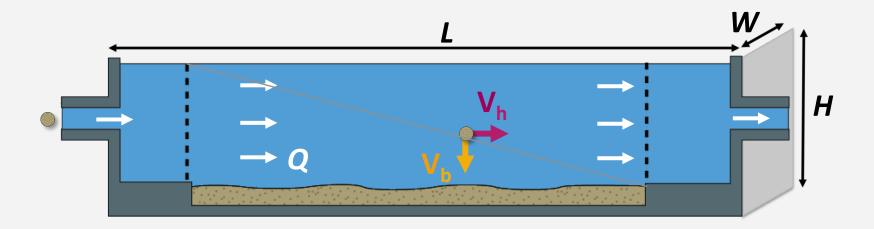


Flocculent settling: hindered settling

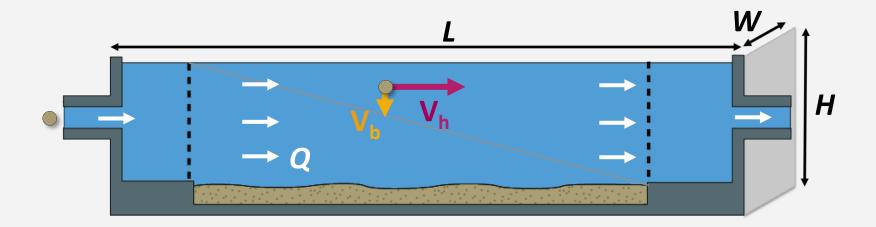


- hindered settling
- transition settling
- compression settling

Discrete settling



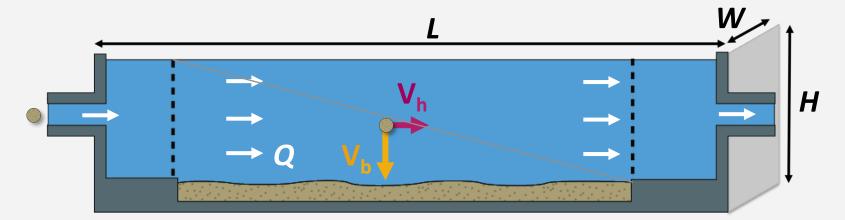
Discrete settling



Discrete settling: Hazen's law

Particle is retained if t_{horizontal} < t_{vertical} , or:

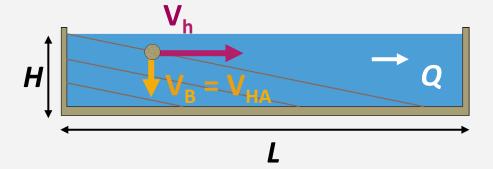
$$\frac{V_b}{H} > \frac{V_h}{L} \quad \textit{and} \quad V_h = \frac{Q}{A_v} = \frac{Q}{H \cdot W} \longrightarrow \frac{V_b}{H} > \frac{Q}{H \cdot W \cdot L} \longrightarrow V_b > \frac{Q}{A_h} = V_{HA}$$



All particles with settling velocity v_b < v_{HA} will be removed

Feed water distributed over H:

• Also particles with $v_b < v_{HA}$ will be removed, in v_b / v_{HA} ratio



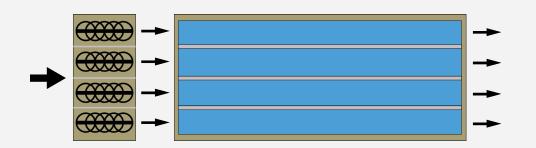
Important design features:

- Sand, surface load: $V_O = V_{HA} = 40 \frac{m^3}{m^2 \cdot h}$ or 0.011 m/s
- $V_h = 0.30 \text{ m/s} = V_S$, Critical 'scouring' or 'slip' velocity

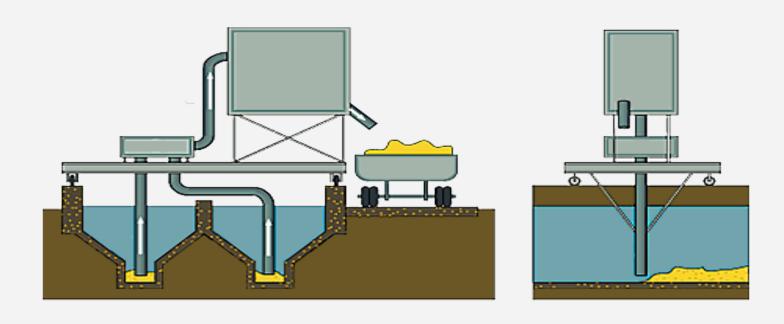


Rectangular grit chamber

- More gutters in parallel
- Long gutter with constant flow rate
- Constant discharge
- Surface loading rate: $V_O = \frac{Q}{A} = 40 \frac{m^3}{m^2 \cdot h}$
- Horizontal velocity V_h, approx. 0.30 m/s
- Length/width ratio: 10:1 15:1

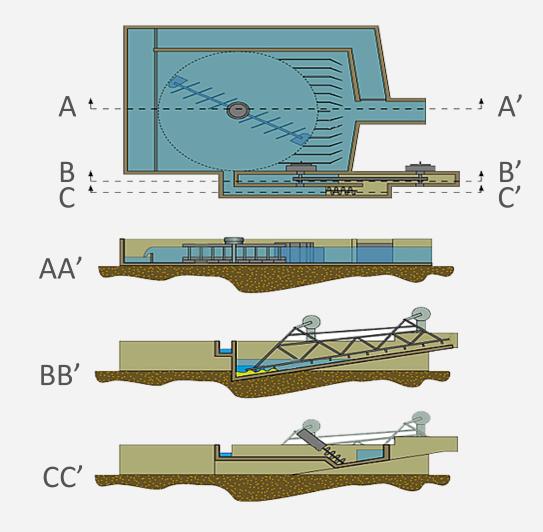


Grit removal: removal of sand with air-lift



Square grit chamber

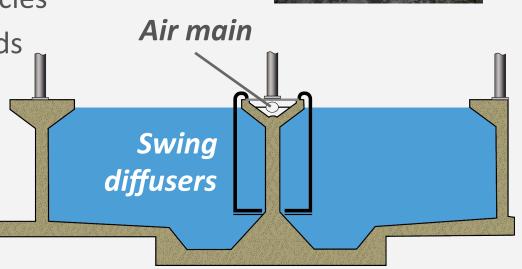
- Water depth 0.8 1m
- Square or circular
- $V_{O, \text{max}} = 30 \frac{\text{m}^3}{\text{m}^2 \cdot \text{h}}$
- Fluctuating discharge
- External sand washer



Aerated grit chamber

- More turbulent
- Increased V_p of sand particles
- Slurry: both sand and solids
- Ex-situ separation

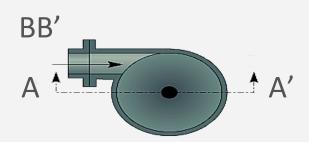




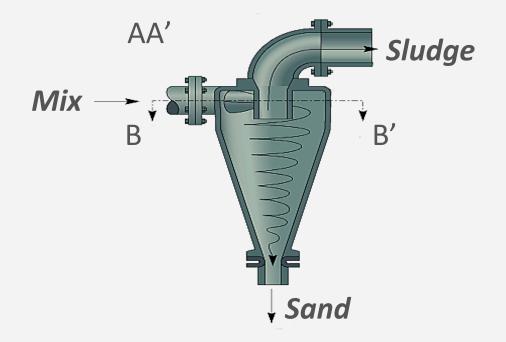
Grit removal: Hydroclone

Slurries:

→ sand – solids separation







Grit removal design

CTB3365x Introduction to water treatment

Prof.dr.ir. Jules B. van Lier

